

Stormwater Management Strategy

Sealed Carpark

The sealed carpark has been designed to grade towards the east and west away from a central high point. Due to site limitations such as grade and limited discharge options, each sub-catchment will have independent above-ground onsite detention (OSD) and stormwater quality treatment.

The eastern portion of the carpark will be graded at approximately 3 - 5% from west to east. The stormwater strategy for the eastern portion of the carpark is as follows:

- Surface runoff will be collected in a series of vegetated swales running the length of the carpark between parking aisles.
- The upstream portion of the vegetated swales will drain to a pit and pipe network designed to convey up to the 100-year storm event. This network will discharge to a wetland located to the east of the proposed aquarium, which will provide the required detention and water quality treatment for this portion of the carpark.
- The downstream portion of the biofiltration swales will discharge into small raingardens located within the aisle medians. A portion of the carpark will discharge to a rain garden to the south of the carpark entry/exit roadway. The raingarden overflow pits will be fitted with orifice plates to provide the required detention storage for this portion of the carpark.
- The carpark entry/exit roadway will discharge directly to the external stormwater network.

The western portion of the carpark has been graded at approximately 4% from east to west. The stormwater strategy for this portion of the carpark can be summarised as follows:

- The majority of pavement runoff will be captured in a series of vegetated swales running the length of the carpark between parking aisles.
- Swale flows will be captured by a pit and pipe network, which will convey stormwater into a biofiltration basin located in open space to the south-west of the carpark.
- The biofiltration basin will discharge to the west via an overflow pit, which will connect into the proposed stormwater network running along the perimeter of the overflow carpark. The outflow pipe from the basin has been sized to convey up to the 2 year ARI storm event, with higher flows allowed to surcharge from a downstream pit and flow overland towards Eastern Creek to the west.

Overflow Carpark

The overflow carpark is located at the south-west corner of the site and grades at approximately 5% to the south-west. The majority of the overflow carpark will be surfaced with turf, using a granular top soil stabilised with geo-grid. A portion of the carpark to the north will be surfaced with asphalt, along with an asphalt access road around the carpark perimeter. The stormwater strategy for the overflow carpark can be summarised as follows:

- Runoff from the unsealed portion of the carpark will be captured in a pit and pipe network located along the perimeter asphalt access road and conveyed to biofiltration treatment and the storage basin.
- Runoff from part of the sealed portion of the overflow carpark will be captured in a swale running along the northern edge of the unsealed portion of the carpark. The remainder of the sealed area will be captured by a pit and pipe network which is conveyed to biofiltration treatment and the storage basin.

Zoo Grounds

The zoo grounds can be split into east and west catchments. Stormwater from the larger western catchment will be directed to the proposed storage basin, which has a minimum total storage volume of 1790 m³. The eastern portion of the site will be directed to a proposed wetland area in the north-east corner of the site, with a minimum volume of 1120 m³. The strategy for each portion of the zoo grounds can be summarised as follows:

- Site runoff from the majority of the site will be captured either by stormwater pits or in vegetated swales and conveyed to biofiltration basins for treatment, prior to discharging into either the wetland or the storage basin.
- Stormwater detention for the western portion of the site will be provided in an open space area immediately adjacent to the proposed storage basin. Detention storage for the eastern portion of the site is provided within the proposed wetland.
- The proposed storage basin will overflow via a piped-outlet during low-intensity storm events, flowing overland towards Eastern Creek. During higher intensity events, the basin will overflow via an emergency spillway into the detention storage area.
- The proposed wetland will discharge via an overflow pit, which discharges to a level spreader at the north-east zoo boundary. For higher intensity storm events, the wetland will discharge over an emergency spillway built into the adjoining access road.
- Roof water runoff from all buildings will be first treated using a first-flush device before discharging into the stormwater network.

Stormwater Harvesting

The stormwater harvesting strategy for the site remains unchanged from that proposed in the previous Stormwater Management Plan submission (Lindsay Dynan, 2015). As such, 80% of the non-potable water demand of the development is to be met using non-potable sources, as previously requested by Blacktown City Council. To confirm the target could still be met with the revised site layout, the MUSIC software package was used to perform a water balance estimate for the site.

The 1790 m³ storage basin at the north-west corner of the site will be the primary non-potable water storage for the site, along with a smaller 500 m³ “Holding Basin” located adjacent to the proposed restaurant building. The revised basin layout incorporates a volume allowance for sediment build-up and minimum pump water levels equal to 20% of the live volume of the basins, as originally requested by Blacktown City Council (Lyndsay Dynan, 2015). The storage volumes provided in the storage and holding basins are provided in Table 1.

Table 1 – Non-potable Water Storage Volumes.

Storage	Live Volume (m³)	20% Settlement Zone Volume (m³)	Total Volume (m³)
Storage Basin	1,490	300	1,790
Holding Basin	420	80	500
Combined	1,910	380	2,290

The estimated non-potable reuse rates for the zoo development have been derived from information provided by the client (Lyndsay Dynan, 2015) and are reproduced in Table 2.

Table 2 – Stormwater Reuse Demands.

Activity	Usage	Equivalent Daily Usage
Irrigation (Tropical)	22.7 L/m ² /week	11,850 L/day
Irrigation (Turf)	22.7 L/m ² /week	17,300 L/day
Toilet Usage	100 L/day/toilet	5,300 L/day
Back of House Hose Down	5 mm/m ² /day	5,750 L/day
Basin/Moat Evaporation	1500 mm/year/m ²	17,150 L/day
TOTALS		
Total Annual Demand (PET Minus Rainfall Distribution)	16,900 kL/year	
Plus Total Daily Demand (Uniform Distribution)	11.05 kL/day	

With the revised storage layout, an estimated reuse efficiency of 80.8% was achieved in MUSIC. This meets Blacktown City Council's requirements for development and is generally in agreement with the reuse efficiency achieved under the original DA documentation.

Onsite Detention (OSD)

As per the previously approved stormwater management plan, OSD measures have been provided to ensure runoff from all portions of the site will be less than or equal to the pre-developed catchment. As described in the previous section, three detention storages have been provided to limit site discharge:

- Within raingardens built into medians in the eastern carpark;
- Within the proposed wetland at the north-eastern corner of the site; and
- Within open space at the north-western corner of the site.

To size the required detention storage and outlet controls, modelling was undertaken using the DRAINS software. In accordance with Blacktown City Council's requirements, OSD measures were designed to limit runoff in the 5 to 100 year ARI storm events, for all storm durations between 5 minutes and 4.5 hours. The adopted ILSAX and antecedent moisture condition (AMC) parameters were in accordance with the Blacktown City Council Drainage Design Manual. The following tables summarise the configuration of the proposed detention measures for each portion of the site and the estimated post-developed runoff rates. For brevity, only the peak outflow for each ARI has been provided here.

Table 3 – Peak Site Discharge for Zoo Grounds West

Contributing Catchment	10.18 ha	
Percentage Impervious	37%	
Storage	Open Space Detention Basin	
Total Storage Volume	1,850 m ³	
ARI	Peak Pre-Developed Discharge (m³/s)	Peak Post-Developed Discharge (m³/s)
5	0.58	0.56
10	1.31	0.85
20	1.72	1.15
50	2.58	1.51
100	3.00	2.07

Table 4 – Peak Site Discharge for Zoo Grounds East

Contributing Catchment	2.75 ha	
Percentage Impervious	40%	
Storage	Wetland	
Total Storage Volume	1,120 m ³	
ARI	Peak Pre-Developed Discharge (m³/s)	Peak Post-Developed Discharge (m³/s)
5	0.23	0.21
10	0.46	0.28
20	0.62	0.32
50	0.94	0.37
100	1.10	0.40

Table 5 – Peak Site Discharge for Sealed Carpark West

Contributing Catchment	0.84 ha	
Percentage Impervious	89%	
Storage	Biofiltration Basin	
Total Storage Volume	190 m ³	
ARI	Peak Pre-Developed Discharge (m³/s)	Peak Post-Developed Discharge (m³/s)
5	0.18	0.17
10	0.28	0.18
20	0.32	0.18
50	0.38	0.18
100	0.43	0.19

Table 6 – Peak Site Discharge for Sealed Carpark East

Contributing Catchment	0.74 ha	
Percentage Impervious	100%	
Storage	Median Raingardens	
Total Storage Volume	190 m ³	
ARI	Peak Pre-Developed Discharge (m³/s)	Peak Post-Developed Discharge (m³/s)
5	0.15	0.12
10	0.24	0.13
20	0.28	0.14
50	0.33	0.16
100	0.37	0.18

Note: A copy of the DRAINS files are available upon request.

Pollutant Load Reduction Targets

The water quality objectives for the site are generally unchanged from the previous stormwater proposal. Stormwater pollutant reduction targets are those required under the BCC 2015 DCP and are reproduced in Table 7.

Pollutant	Treatment Efficiency Target
Total Suspended Solids (TSS)	85% reduction
Total Phosphorous (TP)	65% reduction
Total Nitrogen (TN)	45% reduction
Gross Pollutants (GP)	90% reduction

The proposed treatment measures were analysed using the MUSIC software, using parameters recommended by BCC's 2013 Water Sensitive Urban Design Developer Handbook and the 2010 Draft NSW MUSIC Modelling Guidelines. All MUSIC modelling requirements/assumptions requested by BCC to date have been incorporated into the revised MUSIC model.

Stormwater quality for the eastern portion of the sealed carpark will be achieved through a combination of vegetated swales, raingardens and proprietary pit filter inserts. MUSIC modelling results are provided in Table 8.

Pollutant		Sources	Residual	Reduction (%)
Total Suspended Solids (TSS)	kg/yr.	1140	165	86
Total Phosphorus (TP)	kg/yr.	1.91	0.4	78
Total Nitrogen (TN)	kg/yr.	7.8	3.8	51
Gross Pollutants (GP)	kg/yr.	84	0	100

Primary treatment for the western portion of the carpark will be provided by vegetated swales running the length of the carpark. The swales will discharge into pits fitted with filter inserts to screen gross pollutants. The pit and pipe network will then convey stormwater to a biofiltration basin located in open space at the south-west corner of the carpark. The MUSIC modelling results are provided in Table 9.

Pollutant		Sources	Residual	Reduction (%)
Total Suspended Solids (TSS)	kg/yr.	1970	53	97
Total Phosphorus (TP)	kg/yr.	3.3	0.4	87
Total Nitrogen (TN)	kg/yr.	13.4	4.3	68
Gross Pollutants (GP)	kg/yr.	144	0	100

Table 10 – Zoo Grounds and Unsealed Carpark MUSIC Results.

Pollutant		Sources	Residual	Reduction (%)
Total Suspended Solids (TSS)	kg/yr.	6910	934	87
Total Phosphorus (TP)	kg/yr.	13.2	3.4	75
Total Nitrogen (TN)	kg/yr.	92.1	35.1	62
Gross Pollutants (GP)	kg/yr.	783	18	98

The above results demonstrate that the revised stormwater design is compliant with BCC's water quality objectives and commensurate with the previously submitted stormwater management plan. As a result, the proposed stormwater management plan should have a net-neutral to net-beneficial impact on background water quality from the site and the receiving waterway (Eastern Creek) compared to the previous proposal.

Flooding considerations have been described in the previously submitted Stormwater Management Plan (Lindsay Dynan, 2015). The extent of the development within both the 1 in 100 year ARI and PMF flood events has not been altered.

The proposed stormwater management design presented above has been prepared to comply with Blacktown City Council's Development Control Plan 2015 and associated Design Guidelines, as well as the previously approved DA documentation and industry best practice. The design is therefore compliant with Development Consent Condition C11 (Stormwater Drainage) issued as part of the previously issued Development Consent for the site. Should you have any queries, please do not hesitate to contact the undersigned on (02) 4943 1777.

Reviewed by:

[Signature]

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